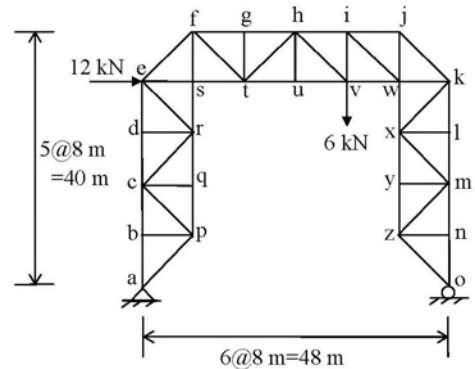


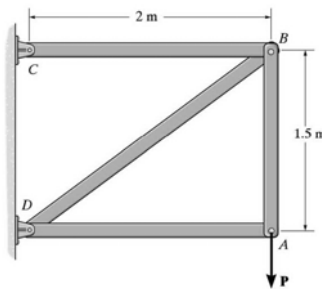
日期：2026 年 06 月 11 日 姓名：_____ 學號：_____

1. 分析如右圖所示桁架結構(各桿件 E 、 A 均相同)，試問：

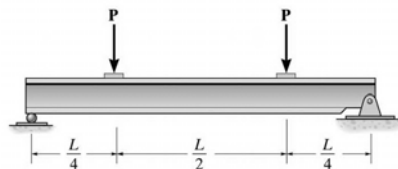
- (1) 各支承反力為何？(6%)
- (2) 零力桿件有幾根？(2%)
- (3) 桿件 jk 、 wk 之受力為何？(6%)
- (4) 最大受力桿件為哪根且大小為何？(6%)



2. 如下圖所示為 A-36 鋼所組成的桁架結構，試求在任何構件均不會產生永久變形下的最大作用力 P 和最大總應變能。(任一構件截面積均為 $2.5 \times 10^3 \text{ mm}^2$ ， $E = 200 \text{ GPa}$ ， $\sigma_y = 250 \text{ MPa}$) (20%)

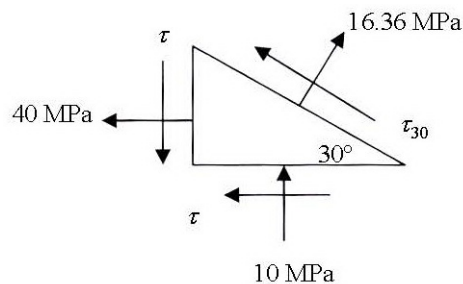


3. 試求如下圖所示樑的彎曲應變能。(20%)



4. 如下圖所示某點之應力狀況，請計算

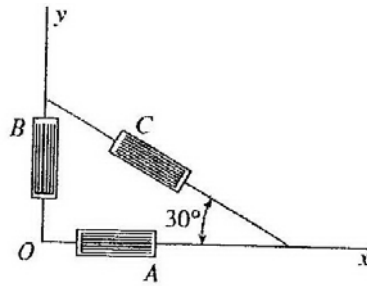
- (1) 未知應力 τ 與 τ_{30} 。(10%)
- (2) 主應力與其所在平面，並繪出相對應元素圖。(10%)



5. 一菊花座應變規貼附於樑上。各量規之讀數分別為 $\varepsilon_A = 1200 \times 10^{-6}$ 、

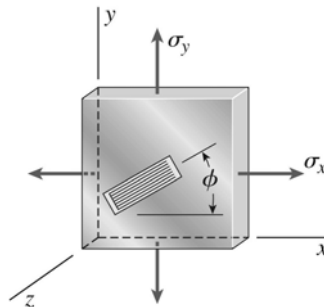
$\varepsilon_B = 200 \times 10^{-6}$ 和 $\varepsilon_C = 200 \times 10^{-6}$ 。試求：

- (1) 平面主應變 (7%) (2) 最大平面剪應變 (7%) (3) 平均正應變 (6%)



6. 一銅板的彈性模數為 $E = 110 \text{ GPa}$ ，蒲松比 $\nu = 0.34$ ，受到 x 與 y 的雙軸應力作用(如下圖)。貼在板上的應變規角度為 $\phi = 35^\circ$ 。若 $\sigma_x = 74 \text{ (MPa)}$ ，量得的應變為 $\varepsilon = 390 \times 10^{-6}$ ，平面內最大剪應力 $(\tau_{\max})_{xy}$ 與最大剪應變 $(\gamma_{\max})_{xy}$ 是多少？

(20%)



7. (1) 上完了一學期的材料力學，對於這門課在學習上有何心得或感想？(5%)
 (2) 對於老師的教學方式或是要如何協助同學們學習好這門課有何建議？(5%)
 (兩題請分開寫，有寫才有分)

(參考公式)

平面應力轉換方程式

$$\sigma_{x'} = \frac{\sigma_x + \sigma_y}{2} + \frac{\sigma_x - \sigma_y}{2} \cos 2\theta + \tau_{xy} \sin 2\theta$$

$$\tau_{x'y'} = -\frac{\sigma_x - \sigma_y}{2} \sin 2\theta + \tau_{xy} \cos 2\theta$$

平面應變轉換方程式

$$\varepsilon_{x'} = \frac{\varepsilon_x + \varepsilon_y}{2} + \frac{\varepsilon_x - \varepsilon_y}{2} \cos 2\theta + \frac{\gamma_{xy}}{2} \sin 2\theta$$

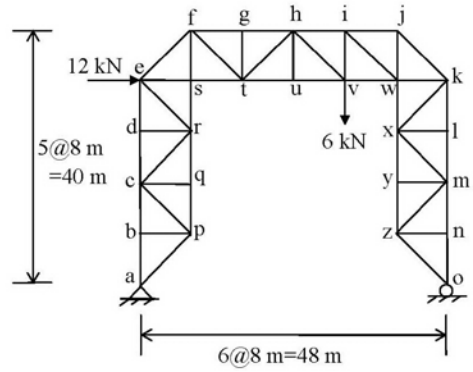
$$\frac{\gamma_{x'y'}}{2} = -\frac{\varepsilon_x - \varepsilon_y}{2} \sin 2\theta + \frac{\gamma_{xy}}{2} \cos 2\theta$$

彎曲公式： $\sigma = -\frac{My}{I}$ ， 剪力公式： $\tau = \frac{VQ}{It}$ ， 扭轉公式： $\tau = \frac{Tc}{J}$

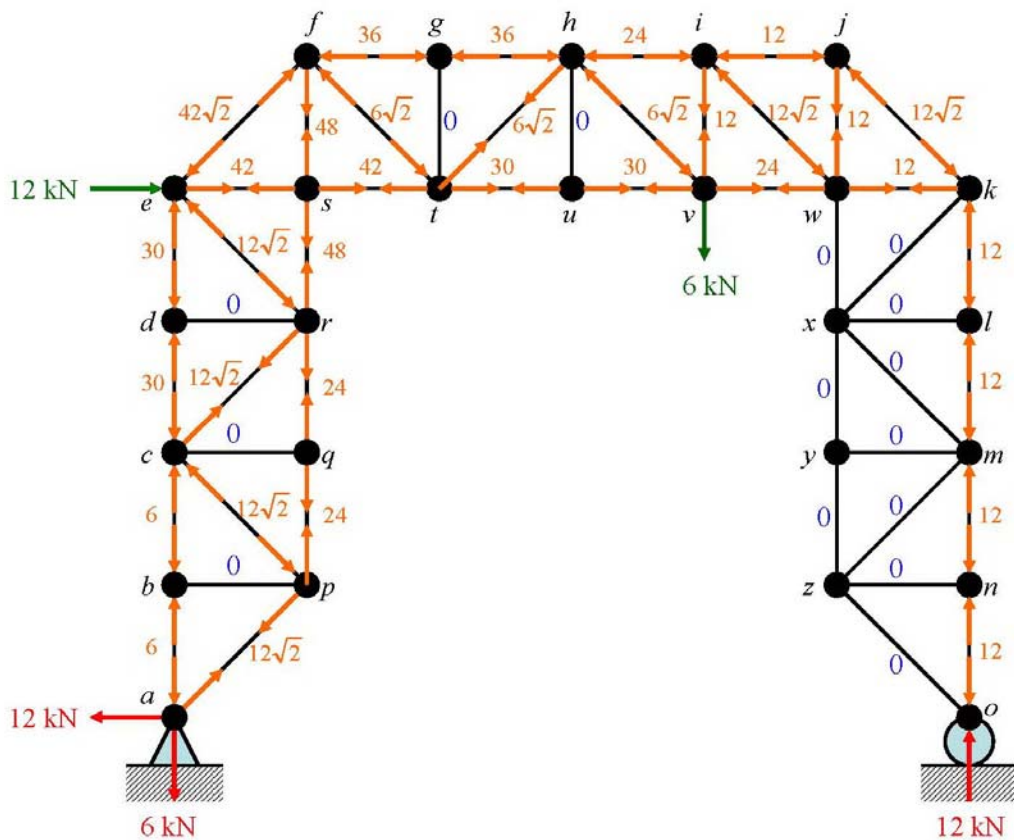
參考解答：

1. 分析如右圖所示桁架結構(各桿件 E、A 均相同)，試問：

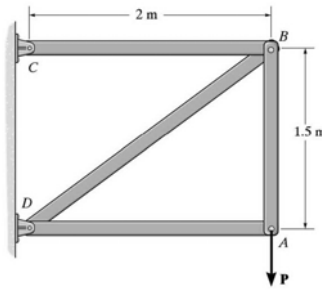
- (1) 各支承反力為何? (6%)
- (2) 零力桿件有幾根? (2%)
- (3) 桿件 jk 、 wk 之受力為何? (6%)
- (4) 最大受力桿件為哪根且大小為何? (6%)



- (1) $a_x = 12$ (kN) (向左), $a_y = 6$ (kN) (向下)
 $o_y = 12$ (kN) (向上)
- (2) 15 根
- (3) $F_{jk} = 12\sqrt{2}$ (kN) (壓), $F_{wk} = 12$ (kN) (拉)
- (4) 最大受力桿件為 ef , $F_{ef} = 42\sqrt{2}$ (kN) (壓)



2. 如下圖所示為 A-36 鋼所組成的桁架結構，試求在任何構件均不會產生永久變形下的最大作用力 P 和最大總應變能。(任一構件截面積均為 $2.5 \times 10^3 \text{ mm}^2$, $E = 200 \text{ GPa}$, $\sigma_Y = 250 \text{ MPa}$) (20%)



$$C_x = \frac{4}{3}P \text{ (向左)}, C_y = 0 \text{ (向左)}$$

$$D_x = \frac{4}{3}P \text{ (向右)}, D_y = P \text{ (向上)}$$

$$\text{可得 } F_{AB} = P \text{ (拉)}, F_{BD} = \frac{5}{3}P \text{ (壓)}, F_{BC} = \frac{4}{3}P \text{ (拉)}, F_{AD} = 0$$

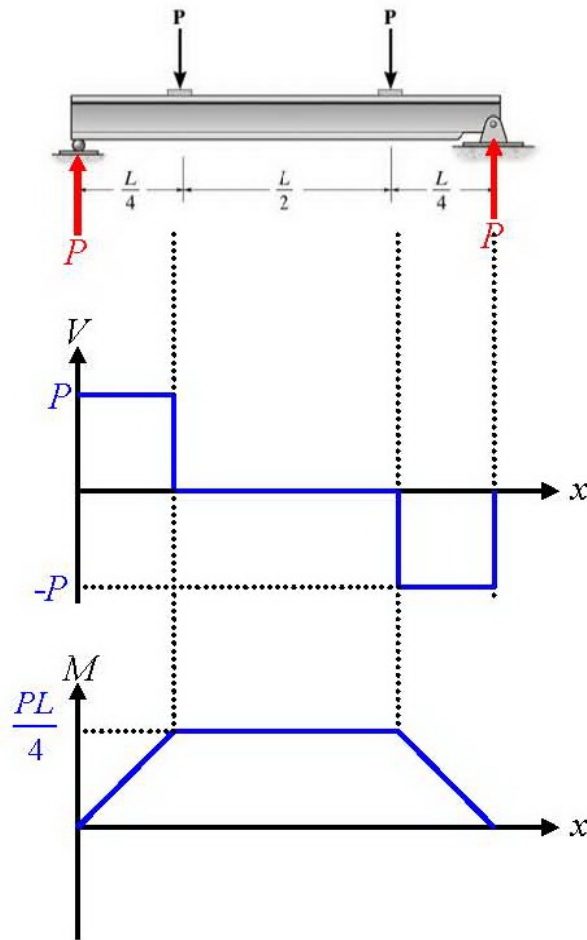
$\therefore$ 最大受力桿件為 BD

$$\sigma_Y = \frac{F_{BD}}{A} \Rightarrow 250 \times 10^6 = \frac{\frac{5}{3}P}{2.5 \times 10^{-3}} \Rightarrow P = 375 \times 10^3 \text{ (N)} = 375 \text{ (kN)}$$

$$\therefore F_{AB} = 375 \text{ (kN)}, F_{BD} = 625 \text{ (kN)}, F_{BC} = 500 \text{ (kN)}$$

$$U_i = \sum \frac{N^2 L}{2AE} = \frac{(375^2 \times 1.5 + 625^2 \times 2.5 + 500^2 \times 2) \times 10^6}{2 \times 2.5 \times 10^{-3} \times 200 \times 10^9} = 1687.5 \text{ (J)} = 1.6875 \text{ (kJ)}$$

3. 試求如下圖所示樑的彎曲應變能。(20%)

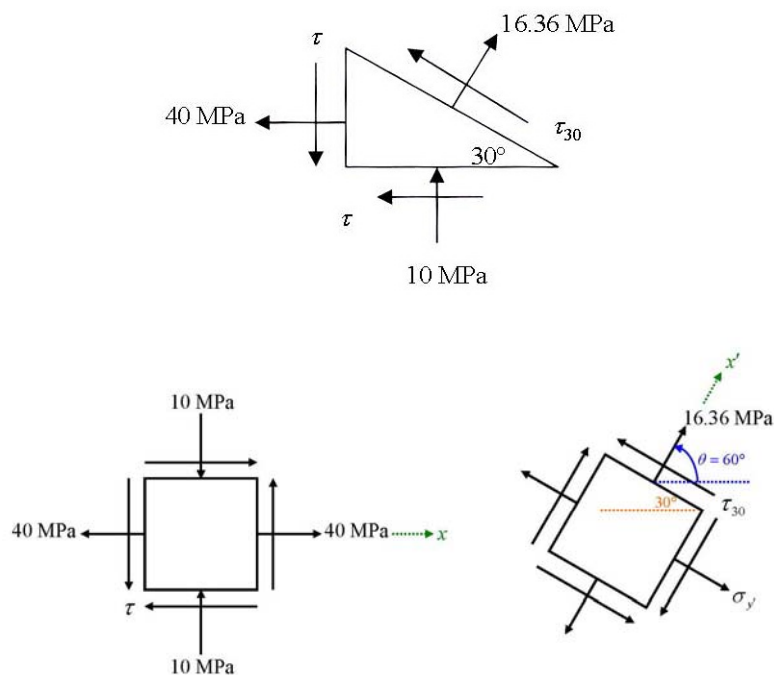


$$\begin{aligned}
 U_i &= \int_0^L \frac{M^2}{2EI} dx = \frac{2}{2EI} \left[\int_0^{\frac{L}{4}} (Px_1)^2 dx_1 + \int_{\frac{L}{4}}^{\frac{L}{2}} \left(\frac{PL}{4}\right)^2 dx_2 \right] \\
 &= \frac{1}{EI} \left[\frac{P^2}{3} \left(\frac{L}{4}\right)^3 + \frac{P^2 L^2}{16} \left(\frac{L}{2} - \frac{L}{4}\right) \right] \\
 &= \frac{P^2 L^3}{48EI}
 \end{aligned}$$

4. 如下圖所示某點之應力狀況，請計算

(1) 未知應力 τ 與 τ_{30} 。(10%)

(2) 主應力與其所在平面，並繪出相對應元素圖。(10%)



(1) 由圖可知： $\sigma_x = 40$ (MPa)， $\sigma_y = -10$ (MPa)， $\tau_{xy} = \tau$ (MPa)， $\theta = 60^\circ$

$$\sigma_{x'} = 16.36 \text{ (MPa)}, \tau_{x'y'} = \tau_{30} \text{ (MPa)}$$

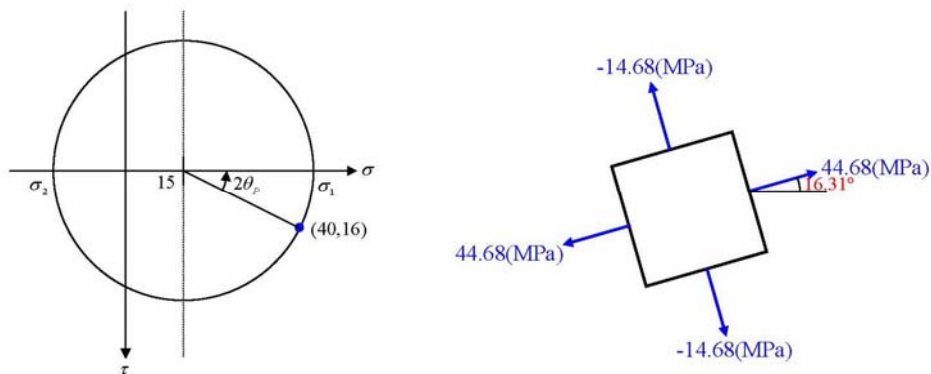
$$\sigma_{x'} = \frac{\sigma_x + \sigma_y}{2} + \frac{\sigma_x - \sigma_y}{2} \cos 2\theta + \tau_{xy} \sin 2\theta$$

$$\Rightarrow 16.36 = \frac{40 + (-10)}{2} + \frac{40 - (-10)}{2} \cos 120^\circ + \tau \cdot \sin 120^\circ \Rightarrow \tau \approx 16 \text{ (MPa)}$$

$$\tau_{x'y'} = -\frac{\sigma_x - \sigma_y}{2} \sin 2\theta + \tau_{xy} \cos 2\theta$$

$$\Rightarrow \tau_{30} = -\frac{40 - (-10)}{2} \sin 120^\circ + 16 \cdot \cos 120^\circ = -29.65 \text{ (MPa)}$$

$$(2) \sigma_{avg} = \frac{\sigma_x + \sigma_y}{2} = 15, \quad O = (\sigma_{avg}, 0) = (15, 0), \quad A = (\sigma_x, \tau_{xy}) = (40, 16)$$



$$R = \sqrt{(40-15)^2 + 16^2} = 29.68$$

$$\sigma_1 = \sigma_{avg} + R = 15 + 29.68 = 44.68 \text{ (MPa)}$$

$$\sigma_2 = \sigma_{avg} - R = 15 - 29.68 = -14.68 \text{ (MPa)}$$

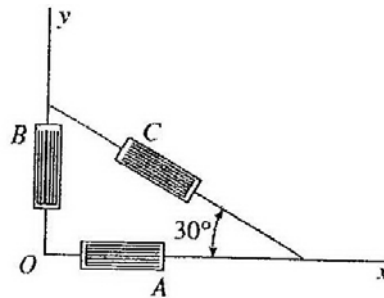
$$\tan 2\theta_p = \frac{16}{40-15} \Rightarrow 2\theta_p = \frac{16}{40-15} = 0.5693 \text{ (rad)} = 32.62^\circ$$

$$\Rightarrow \theta_p = 16.31^\circ \text{ (逆)}$$

5. 一菊花座應變規貼附於樑上。各量規之讀數分別為 $\varepsilon_A = 1200 \times 10^{-6}$ 、

$\varepsilon_B = 200 \times 10^{-6}$ 和 $\varepsilon_C = 200 \times 10^{-6}$ 。試求：

(1) 平面主應變 (7%) (2) 最大平面剪應變 (7%) (3) 平均正應變 (6%)



$$\varepsilon_{x'} = \frac{\varepsilon_x + \varepsilon_y}{2} + \frac{\varepsilon_x - \varepsilon_y}{2} \cos 2\theta + \frac{\gamma_{xy}}{2} \sin 2\theta$$

$$\varepsilon_A = 1200 \times 10^{-6} = \frac{\varepsilon_x + \varepsilon_y}{2} + \frac{\varepsilon_x - \varepsilon_y}{2} \cos 0^\circ + \frac{\gamma_{xy}}{2} \sin 0^\circ \Rightarrow \varepsilon_x = 1200 \times 10^{-6}$$

$$\varepsilon_B = 200 \times 10^{-6} = \frac{\varepsilon_x + \varepsilon_y}{2} + \frac{\varepsilon_x - \varepsilon_y}{2} \cos 180^\circ + \frac{\gamma_{xy}}{2} \sin 180^\circ \Rightarrow \varepsilon_y = 200 \times 10^{-6}$$

$$\varepsilon_C = 200 \times 10^{-6} = \frac{\varepsilon_x + \varepsilon_y}{2} + \frac{\varepsilon_x - \varepsilon_y}{2} \cos 300^\circ + \frac{\gamma_{xy}}{2} \sin 300^\circ$$

$$\Rightarrow \gamma_{xy} = 1000\sqrt{3} \times 10^{-6} = 1732 \times 10^{-6}$$

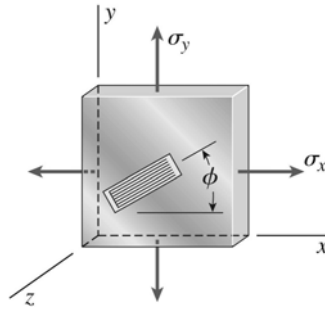
$$(1) \quad \varepsilon_{1,2} = \frac{\varepsilon_x + \varepsilon_y}{2} \pm \sqrt{\left(\frac{\varepsilon_x - \varepsilon_y}{2}\right)^2 + \left(\frac{\gamma_{xy}}{2}\right)^2} = 1700 \times 10^{-6} \text{ and } -300 \times 10^{-6}$$

$$(2) \quad \frac{\gamma_{\max}}{2} = \sqrt{\left(\frac{\varepsilon_x - \varepsilon_y}{2}\right)^2 + \left(\frac{\gamma_{xy}}{2}\right)^2} = 1000 \times 10^{-6} \Rightarrow \gamma_{\max} = 2000 \times 10^{-6}$$

$$(3) \quad \varepsilon_{avg} = \frac{\varepsilon_1 + \varepsilon_2}{2} = 700 \times 10^{-6}$$

6. 一銅板的彈性模數為 $E = 110 \text{ GPa}$ ，蒲松比 $\nu = 0.34$ ，受到 x 與 y 的雙軸應力作用(如下圖)。貼在板上的應變規角度為 $\phi = 35^\circ$ 。若 $\sigma_x = 74 \text{ (MPa)}$ ，量得的應變為 $\varepsilon = 390 \times 10^{-6}$ ，平面內最大剪應力 $(\tau_{\max})_{xy}$ 與最大剪應變 $(\gamma_{\max})_{xy}$ 是多少？

(20%)



$$\sigma_x = 74 \text{ MPa}, \quad \phi = 35^\circ, \quad \varepsilon_{x'} = \varepsilon = 390 \times 10^{-6}, \quad \tau_{xy} = 0 \Rightarrow \gamma_{xy} = 0$$

$$\varepsilon_x = \frac{1}{E}(\sigma_x - \nu\sigma_y), \quad \varepsilon_y = \frac{1}{E}(\sigma_y - \nu\sigma_x)$$

$$\varepsilon_{x'} = \frac{\varepsilon_x + \varepsilon_y}{2} + \frac{\varepsilon_x - \varepsilon_y}{2} \cos 2\theta + \frac{\gamma_{xy}}{2} \sin 2\theta$$

$$\Rightarrow 390 \times 10^{-6} = \frac{1-\nu}{2E}(\sigma_x + \sigma_y) + \frac{1+\nu}{2E}(\sigma_x - \sigma_y) \cos 70^\circ$$

$$\Rightarrow 390 \times 10^{-6} \cdot 2 \cdot 110 \cdot 10^3 = 0.66 \cdot (74 + \sigma_y) + 1.34 \cdot (74 - \sigma_y) \cdot 0.342$$

$$\Rightarrow 3.05 = 0.66\sigma_y - 0.4583\sigma_y$$

$$\Rightarrow \sigma_y = 15.12 \text{ (MPa)}$$

$$(\tau_{\max})_{xy} = \frac{\sigma_x - \sigma_y}{2} = 29.44 \text{ (MPa)}$$

$$G = \frac{E}{2(1+\nu)} = 41.045 \text{ (GPa)}$$

$$(\gamma_{\max})_{xy} = \frac{(\tau_{\max})_{xy}}{G} = \frac{29.44}{41.045 \cdot 10^3} = 7.17 \cdot 10^{-4}$$