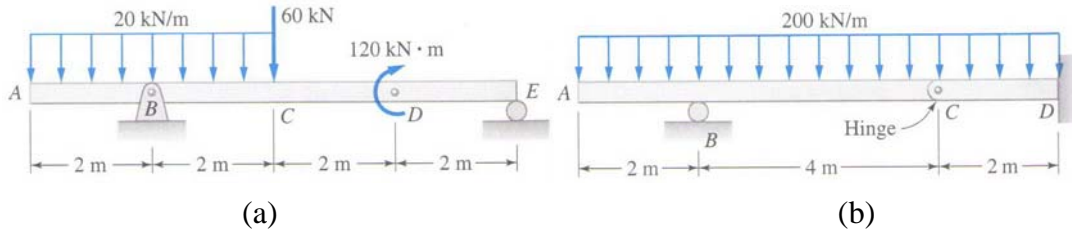
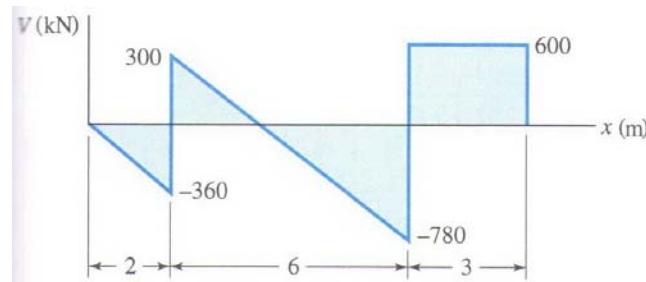


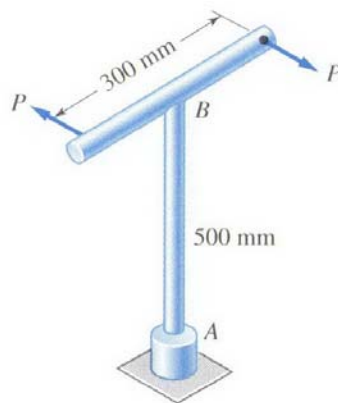
1. 請畫出下面各梁的剪力圖和彎矩圖。(20%)



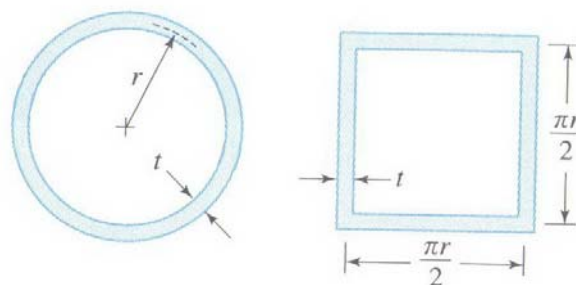
2. 依照所給的剪力圖，繪製相對應的負荷圖與彎矩圖。假設並無力偶作用於梁上。(10%)



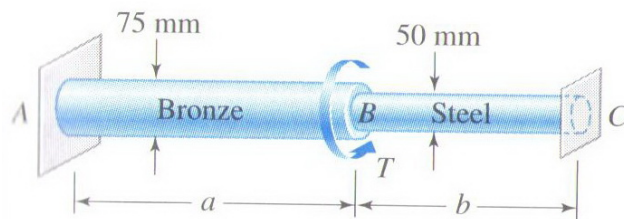
3. 大小為  $P$  的二力作用於下圖所示的鉸手上。已知鋼軸  $AB$  的直徑是 15 mm。如軸內之剪應力不可超過 120 MPa 且扭轉角不可超過  $5^\circ$ 。試求最大容許  $P$  值。(鋼的  $G = 80 \text{ GPa}$ ) (15%)



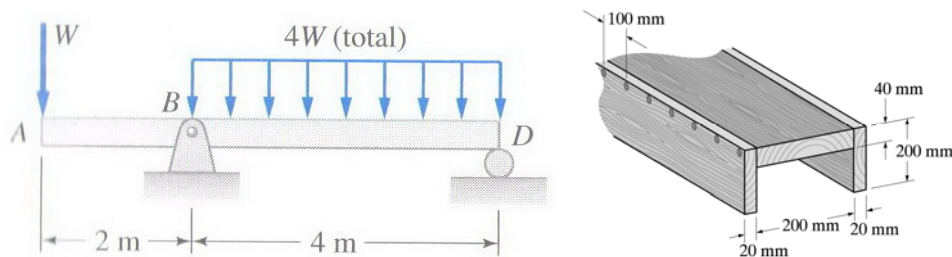
4. 兩相同的金屬平版分別被製成如下圖所示之圓形及方形橫截面管件。若兩支管件都受到相同的扭矩，試求：
- (1) 剪應力  $\tau_{\text{圓形}} / \tau_{\text{方形}}$  之比值。(10%)
  - (2) 扭轉角  $\phi_{\text{圓形}} / \phi_{\text{方形}}$  之比值。(10%)



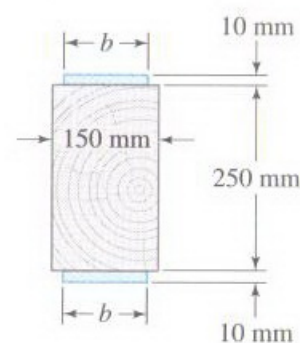
5. 如下圖所示，複合軸兩端固定於剛性壁。其中青銅段  $AB$  直徑為 75 mm 及  $G = 35 \text{ GPa}$ 。而鋼段  $BC$  的直徑為 50 mm 及  $G = 83 \text{ GPa}$ 。
- (1) 已知  $a = 2 \text{ m}$  和  $b = 1.5 \text{ m}$ 。若青銅(Bronze)與鋼(Steel)之最大剪應力分別不超過 60 MPa 和 80 MPa，試求所能施加之最大扭矩  $T$ 。(10%)
- (2) 若使各材料的應力達其容許極限，試問扭矩  $T$  和  $b/a$  應為多少？(10%)



6. 樑承受集中負荷  $W$  和總合力  $4W$  之均佈負荷如下左圖，其斷面如下右圖所示。
- (1) 此樑斷面的中性軸位置？(4%)
- (2) 此斷面中性軸的面積慣性矩  $I = ?$  (4%)
- (3) 此樑的最大拉應力和最大壓應力分別為何？(8%)
- (4) 若容許拉伸應力為 60 MPa，容許壓縮應力為 100 MPa，試求  $W$  之最大容許值。(4%)



7. 木樑(wood)在底和頂端以寬度  $b$  的鋼板(steel)強化。若木材的容許應力為 10 MPa，鋼為 120 MPa 且彈性模數比為  $E_{st}/E_{wd} = 15$ ，試求能承受  $40 \text{ kN} \cdot \text{m}$  彎矩的最小寬度  $b$ 。(15%)



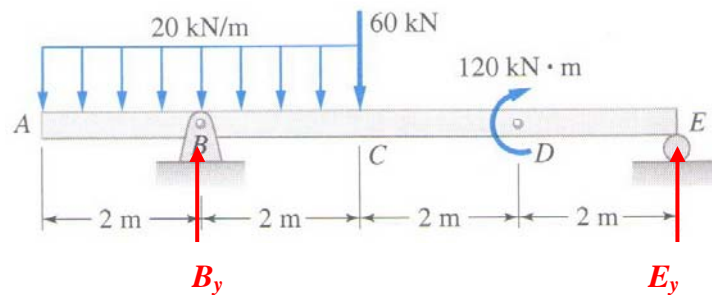
$$\text{薄壁樑扭轉公式: } \tau_{avg} = \frac{T}{2A_m}$$

$$\text{薄壁樑扭轉角公式: } \phi = \frac{TL}{4A_m^2 G} \oint \frac{ds}{t}$$

參考解答:

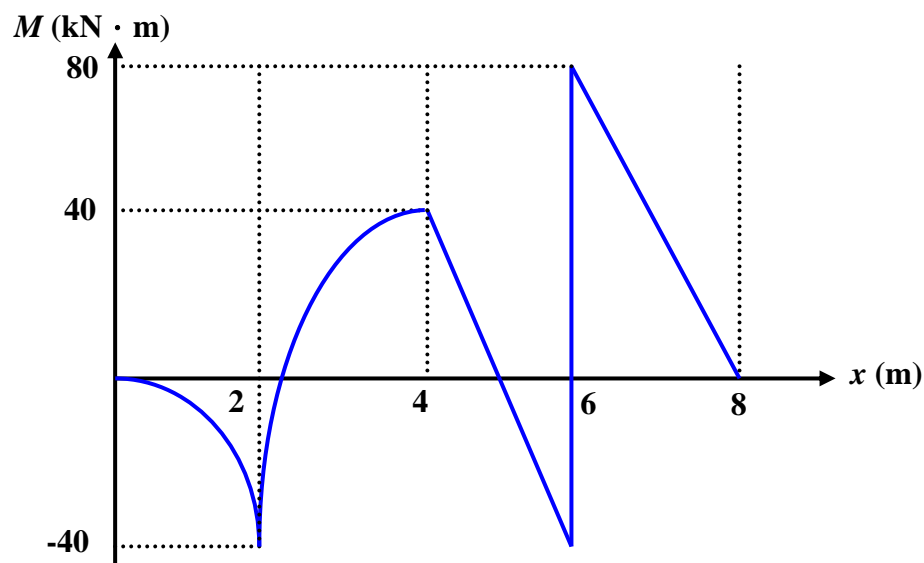
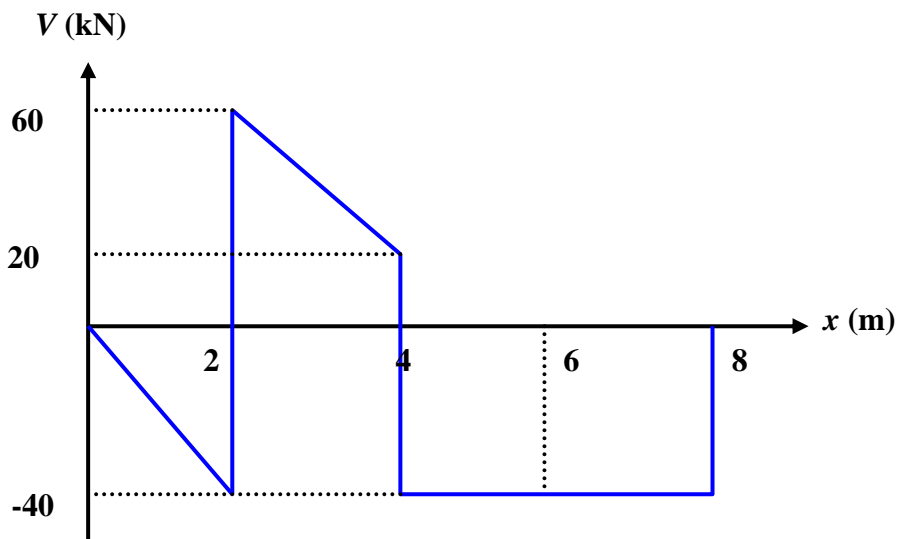
1. 請畫出下面各樑的的剪力圖和彎矩圖。(20%)

(a)

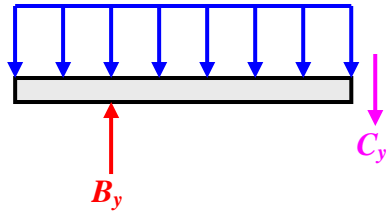
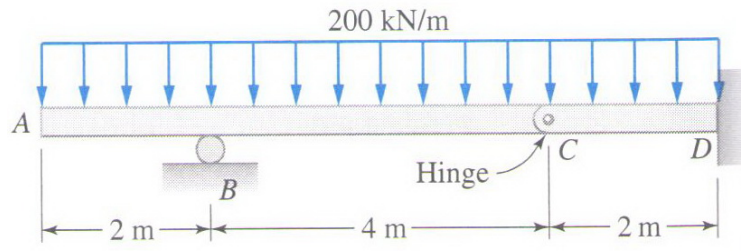


$$\sum M_E = 0 \Rightarrow B_y \cdot 6 - (20 \cdot 4) \cdot 6 - 60 \cdot 4 + 120 = 0 \Rightarrow B_y = 100 \text{ (kN)}$$

$$\sum F_y = 0 \Rightarrow B_y + E_y - 20 \cdot 4 - 60 = 0 \Rightarrow E_y = 40 \text{ (kN)}$$

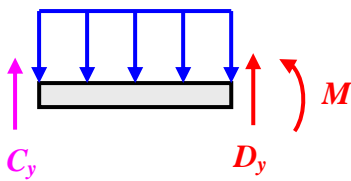


(b)



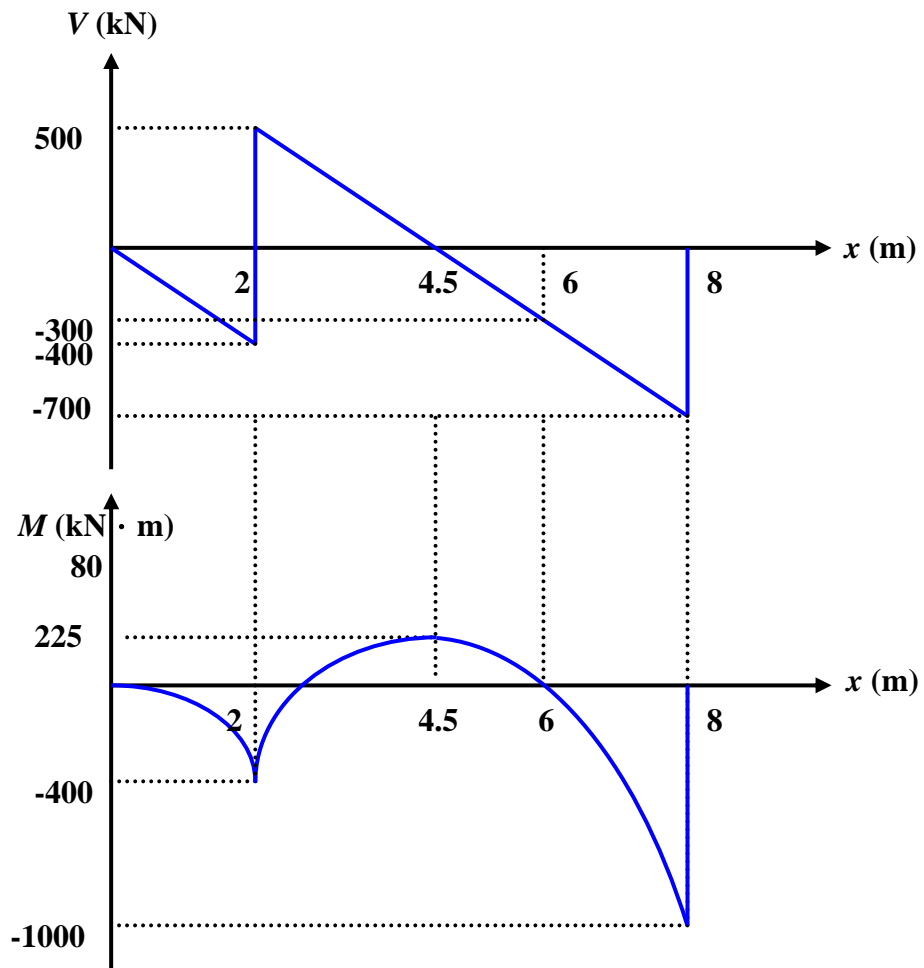
$$\begin{aligned}\sum M_C = 0 &\Rightarrow B_y \cdot 4 - (200 \cdot 6) \cdot 3 = 0 \\ &\Rightarrow B_y = 900 \text{ (kN)}\end{aligned}$$

$$\sum F_y = 0 \Rightarrow C_y = -300 \text{ (kN)}$$

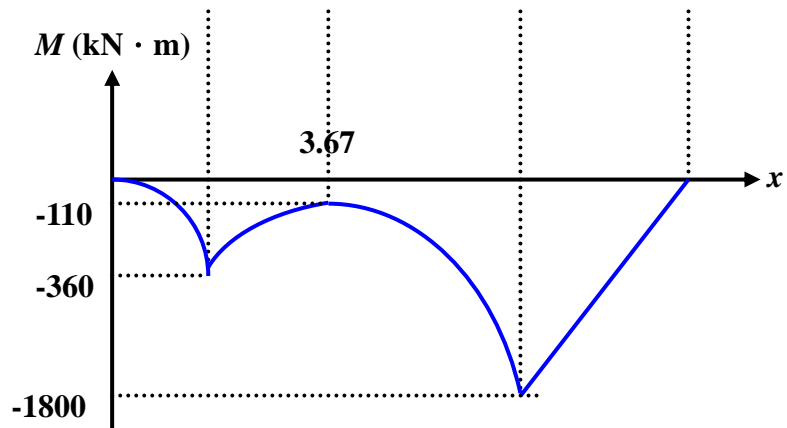
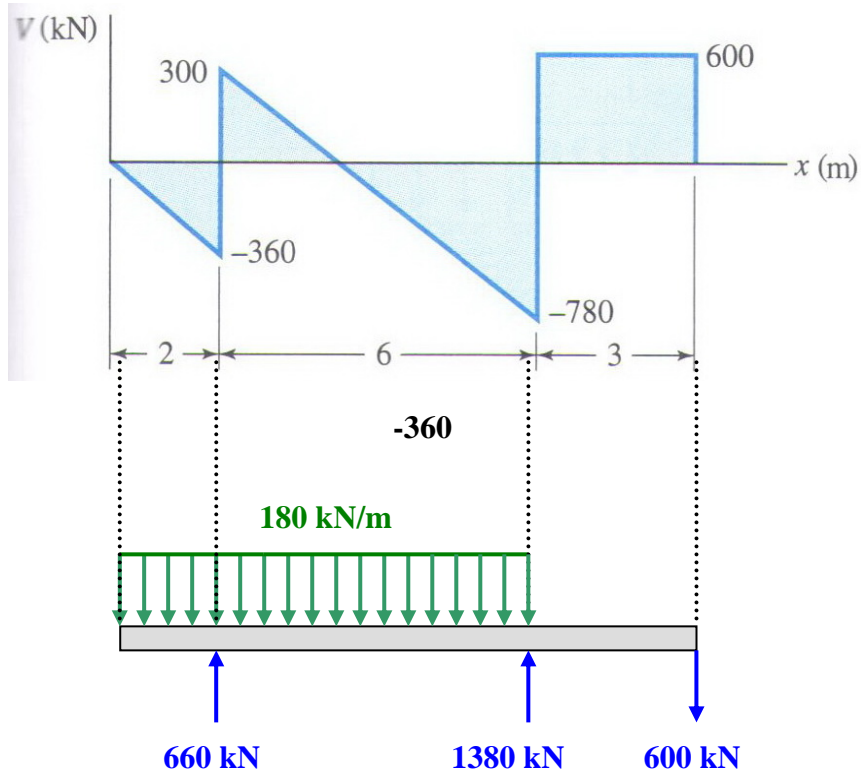


$$\sum F_y = 0 \Rightarrow D_y = 700 \text{ (kN)}$$

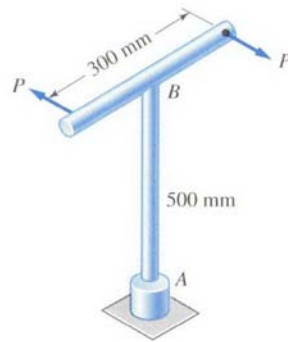
$$\begin{aligned}\sum M_D = 0 &\Rightarrow M - C_y \cdot 2 + (200 \cdot 2) \cdot 1 = 0 \\ &\Rightarrow M = -1000 \text{ (kN} \cdot \text{m)}\end{aligned}$$



2. 依照所給的剪力圖，繪製相對應的負荷圖與彎矩圖。假設並無力偶作用於梁上。(10%)



3. 大小為  $P$  的二力作用於下圖所示的扳手上。已知鋼軸  $AB$  的直徑是 15 mm。如軸內之剪應力不可超過 120 MPa 且扭轉角不可超過  $5^\circ$ 。試求最大容許  $P$  值。(鋼的  $G = 80 \text{ GPa}$ ) (15%)



$$T = 0.3P$$

$$J = \frac{\pi r^4}{2} = \frac{\pi \left(\frac{d}{2}\right)^4}{2} = \frac{\pi d^4}{32}$$

$$\tau = \frac{Tc}{J} = \frac{T \cdot \left(\frac{d}{2}\right)}{\frac{\pi d^4}{32}} = \frac{16T}{\pi d^3} \Rightarrow 120 \cdot 10^6 = \frac{16 \cdot (0.3P)}{\pi \cdot (0.015)^3} \Rightarrow P = 265.07 \text{ (N)}$$

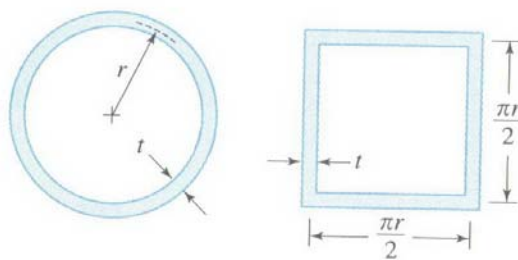
$$\phi = \frac{TL}{JG} \Rightarrow 5 \cdot \frac{\pi}{180} = \frac{(0.3P) \cdot 0.5}{\frac{\pi(0.015)^4}{32} \cdot 80 \cdot 10^9} \Rightarrow P = 231.32 \text{ (N)}$$

$\therefore P$  為兩者取較小者，即  $P = 231.32 \text{ (N)}$

4. 兩相同的金屬平版分別被製成如下圖所示之圓形及方形橫截面管件。若兩支管件都受到相同的扭矩，試求：

(1) 剪應力  $\tau_{\text{圓形}} / \tau_{\text{方形}}$  之比值。(10%)

(2) 扭轉角  $\phi_{\text{圓形}} / \phi_{\text{方形}}$  之比值。(10%)



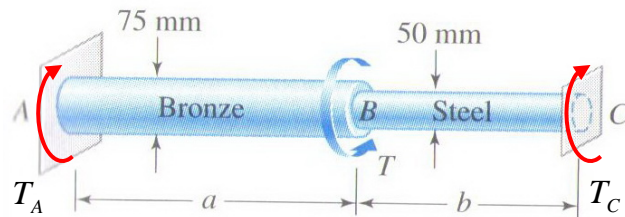
$$(1) \tau = \frac{T}{2A_m} \Rightarrow \frac{\tau_{\text{圓形}}}{\tau_{\text{方形}}} = \frac{(A_m)_{\text{square}}}{(A_m)_{\text{circle}}} = \frac{\left(\frac{\pi r}{2}\right)^2}{\pi r^2} = \frac{\pi}{4}$$

$$(2) \phi = \frac{TL}{4A_m^2 G} \oint \frac{ds}{t} \Rightarrow \frac{\tau_{\text{圓形}}}{\tau_{\text{方形}}} = \frac{(A_m^2)_{\text{square}} \cdot \left(\oint ds\right)_{\text{circle}}}{(A_m^2)_{\text{circle}} \cdot \left(\oint ds\right)_{\text{square}}} = \frac{\left(\frac{\pi r}{2}\right)^4 \cdot 2\pi r}{(\pi r^2)^2 \cdot \frac{\pi r}{2} \cdot 4} = \frac{\pi^2}{16}$$

5. 如下圖所示，複合軸兩端固定於剛性壁。其中青銅段  $AB$  直徑為 75 mm 及  $G = 35 \text{ GPa}$ 。而鋼段  $BC$  的直徑為 50 mm 及  $G = 83 \text{ GPa}$ 。

(1) 已知  $a = 2 \text{ m}$  和  $b = 1.5 \text{ m}$ 。若青銅(Bronze)與鋼(Steel)之最大剪應力分別不超過 60 MPa 和 80 MPa，試求所能施加之最大扭矩  $T$ 。(10%)

(2) 若使各材料的應力達其容許極限，試問扭矩  $T$  和  $b/a$  應為多少？(10%)



(1) 由扭矩平衡可得:  $T_A + T_C = T$

$$\begin{aligned} \text{由相容方程 } \phi_{B/A} = -\phi_{C/B} &\Rightarrow \frac{T_A a}{J_{AB} G_{br}} = -\frac{(-T_C) b}{J_{BC} G_{st}} \\ &\Rightarrow T_A = \frac{b J_{AB} G_{br}}{a J_{BC} G_{st}} T_C \end{aligned}$$

$$\therefore \frac{b J_{AB} G_{br}}{a J_{BC} G_{st}} T_C + T_C = T \Rightarrow \frac{1.5 \cdot 75^4 \cdot 35}{2 \cdot 50^4 \cdot 83} T_C + T_C = T$$

$$\Rightarrow T_C = \frac{T}{2.6011} = 0.3845T$$

$$\Rightarrow T_A = 0.6155T$$

$$\tau_{br} = \frac{T_A \cdot c_{AB}}{J_{AB}} \Rightarrow 60 \cdot 10^6 = \frac{0.6155T \cdot (\frac{75}{2} \cdot 10^{-3})}{\frac{\pi}{2} \cdot (\frac{75}{2} \cdot 10^{-3})^4}$$

$$\Rightarrow T = 8.0749 \cdot 10^3 \text{ (N} \cdot \text{m)} = 8.0749 \cdot 10^3 \text{ (kN} \cdot \text{m)}$$

$$\tau_{st} = \frac{T_C \cdot c_{BC}}{J_{BC}} \Rightarrow 80 \cdot 10^6 = \frac{0.3845T \cdot (\frac{50}{2} \cdot 10^{-3})}{\frac{\pi}{2} \cdot (\frac{50}{2} \cdot 10^{-3})^4}$$

$$\Rightarrow T = 5.1066 \cdot 10^3 \text{ (N} \cdot \text{m)} = 5.1066 \cdot 10^3 \text{ (kN} \cdot \text{m)}$$

$\therefore$  能施加之最大扭矩  $T = 5.1066 \cdot 10^3 \text{ (kN} \cdot \text{m)}$

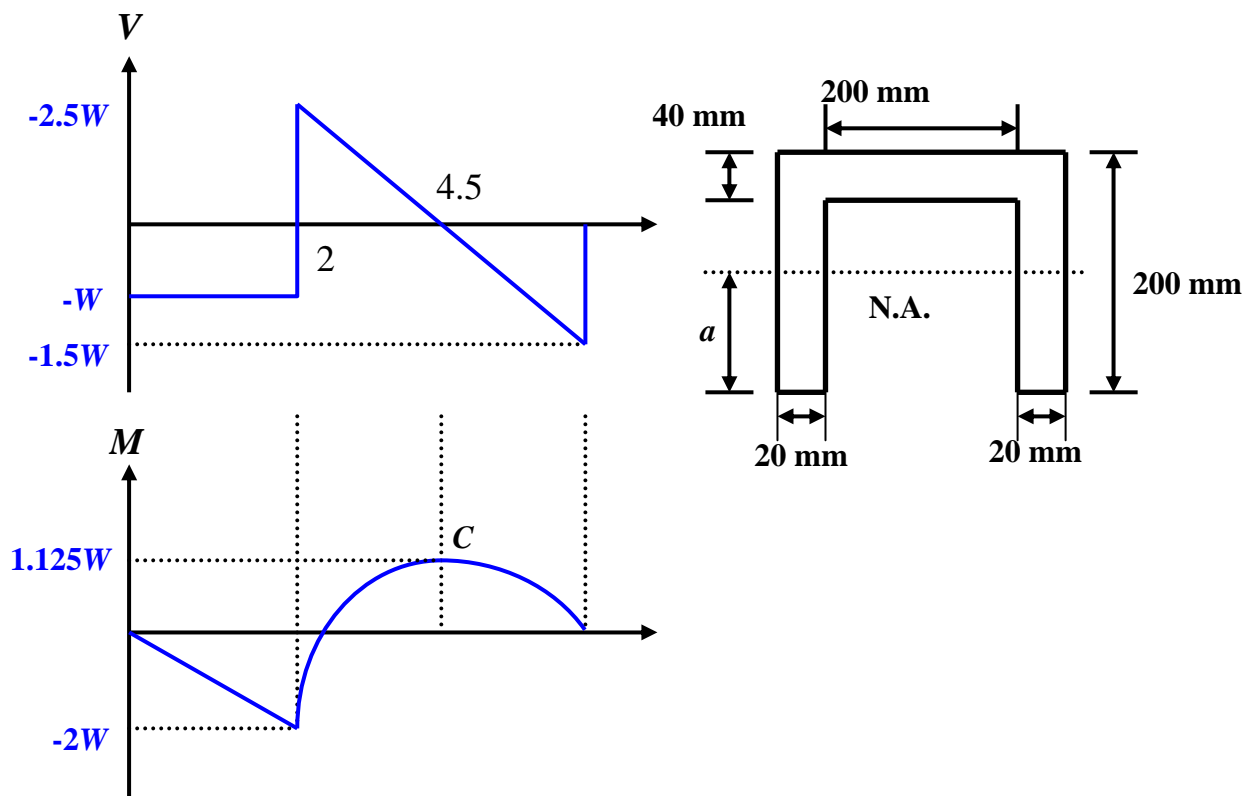
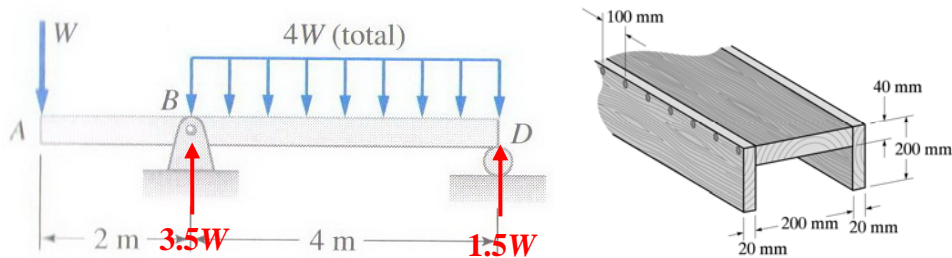
(2) 由  $\tau_{\max} = \frac{T \cdot c}{J}$  與  $\phi = \frac{TL}{JG}$  可得  $\phi = \frac{\tau_{\max} \cdot L}{c \cdot G}$

$$\begin{aligned} \text{由相容方程 } \phi_{B/A} = -\phi_{C/B} &\Rightarrow \frac{(\tau_{\max})_{br} a}{c_{AB} G_{br}} = \frac{(\tau_{\max})_{st} b}{c_{BC} G_{st}} \\ &\Rightarrow \frac{b}{a} = \frac{(\tau_{\max})_{br}}{(\tau_{\max})_{st}} \cdot \frac{c_{BC}}{c_{AB}} \cdot \frac{G_{st}}{G_{br}} = \frac{60}{80} \cdot \frac{50}{75} \cdot \frac{83}{35} \\ &\Rightarrow \frac{b}{a} = 1.1857 \end{aligned}$$

$$\begin{aligned}
 T = T_A + T_C &= \frac{(\tau_{\max})_{AB} \cdot J_{AB}}{c_{AB}} + \frac{(\tau_{\max})_{BC} \cdot J_{BC}}{c_{BC}} \\
 &= \frac{60 \cdot 10^6 \cdot \frac{\pi}{2} \left(\frac{75}{2} \cdot 10^{-3}\right)^4}{\frac{75}{2} \cdot 10^{-3}} + \frac{80 \cdot 10^6 \cdot \frac{\pi}{2} \left(\frac{50}{2} \cdot 10^{-3}\right)^4}{\frac{50}{2} \cdot 10^{-3}} \\
 &= 6.9336 \cdot 10^3 \text{ (N} \cdot \text{m)} \\
 &= 6.9336 \text{ (kN} \cdot \text{m)}
 \end{aligned}$$

6. 樑承受集中負荷  $W$  和總合力  $4W$  之均佈負荷如下左圖，其斷面如下右圖所示。

- (1) 此樑斷面的中性軸位置？ (4%)
- (2) 此斷面中性軸的面積慣性矩  $I = ?$  (4%)
- (3) 此樑的最大拉應力和最大壓應力分別為何？ (8%)
- (4) 若容許拉伸應力為 60 MPa，容許壓縮應力為 100 MPa，試求  $W$  之最大容許值。 (4%)





$$(1) \bar{y} = \frac{\sum y_i A_i}{\sum A_i} = \frac{0.18 \cdot (0.2 \cdot 0.04) + 0.1 \cdot (0.02 \cdot 0.2) \cdot 2}{(0.2 \cdot 0.04) + (0.02 \cdot 0.2) \cdot 2} = 0.14 \text{ (m)}$$

$$\therefore a = \bar{y} = 0.14 \text{ (m)}$$

$$(2) I = \left[ \frac{1}{12} \cdot 0.02 \cdot 0.2^3 + 0.02 \cdot 0.2 \cdot (0.14 - 0.1)^2 \right] \cdot 2 \\ + \frac{1}{12} \cdot 0.2 \cdot 0.04^3 + 0.2 \cdot 0.04 \cdot (0.18 - 0.14)^2 \\ \Rightarrow I = \frac{160}{3} \cdot 10^{-6} = 53.3333 \cdot 10^{-6} \text{ (m}^4\text{)}$$

$$(3) \sigma = -\frac{My}{I}$$

$$(\sigma_B)_{top} = -\frac{(-2W) \cdot 0.06}{\frac{160}{3} \cdot 10^{-6}} = \frac{9W}{4} \cdot 10^3 \text{ (Pa)} = 2.25W \text{ (kPa)}$$

$$(\sigma_B)_{bottom} = -\frac{(-2W) \cdot (-0.14)}{\frac{160}{3} \cdot 10^{-6}} = -\frac{21W}{4} \cdot 10^3 \text{ (Pa)} = -5.25W \text{ (kPa)}$$

$$(\sigma_C)_{top} = -\frac{1.125W \cdot 0.06}{\frac{160}{3} \cdot 10^{-6}} = -\frac{81W}{64} \cdot 10^3 \text{ (Pa)} = -1.2656W \text{ (kPa)}$$

$$(\sigma_C)_{bottom} = -\frac{1.125 \cdot (-0.14)}{\frac{160}{3} \cdot 10^{-6}} = \frac{189W}{64} \cdot 10^3 \text{ (Pa)} = 2.9531W \text{ (kPa)}$$

此樑的最大壓應力在  $B$  點底部為  $5.25W$  kPa

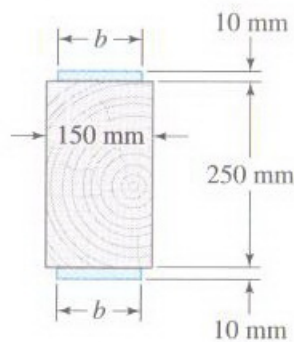
最大拉應力在  $C$  點底部為  $2.9531W$  MPa

$$(4) -100 \cdot 10^6 = -\frac{21W}{4} \cdot 10^3 \Rightarrow W = 19.0476 \cdot 10^3 \text{ (N)} = 19.0476 \text{ (kN)}$$

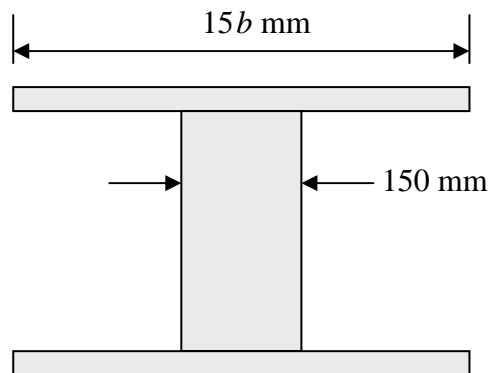
$$60 \cdot 10^6 = \frac{189W}{64} \cdot 10^3 \Rightarrow W = 20.3175 \cdot 10^3 \text{ (N)} = 20.3175 \text{ (kN)}$$

$$\therefore W_{\max} = 19.0476 \text{ (kN)}$$

7. 木樑(wood)在底和頂端以寬度  $b$  的鋼板(steel)強化。若木材的容許應力為 10 MPa，鋼為 120 MPa 且彈性模數比為  $E_{st}/E_{wd} = 15$ ，試求能承受 40 kN·m 彎矩的最小寬度  $b$ 。(15%)



將鋼轉換成木頭，可知轉換因子  $n = \frac{E_{st}}{E_{wd}} = 15$



$$I = \left[ \frac{1}{12} 15b \cdot 10^3 + (15b \cdot 10) \cdot 130^2 \right] \cdot 2 + \frac{1}{12} 150 \cdot 250^3$$

$$= (5.0725b + 195.3125) \cdot 10^6 \text{ (mm}^4\text{)}$$

$$= (5.0725b + 195.3125) \cdot 10^{-6} \text{ (m}^4\text{)}$$

$$\sigma_{wd} = \frac{M c_{wd}}{I} \Rightarrow 10 \cdot 10^6 = \frac{40 \cdot 10^3 \cdot (125 \cdot 10^{-3})}{(5.0725b + 195.3125) \cdot 10^{-6}} \Rightarrow b = 60.07 \text{ (mm)}$$

$$\sigma_{st} = n \sigma'_{st} = \frac{n M c_{st}}{I} \Rightarrow 120 \cdot 10^6 = \frac{15 \cdot 40 \cdot 10^3 \cdot (135 \cdot 10^{-3})}{(5.0725b + 195.3125) \cdot 10^{-6}}$$

$$\Rightarrow b = 94.57 \text{ (mm)}$$

$\therefore$  最小寬度  $b$  為 94.57 mm